

Peninsula Consulting Coastal Structural Engineers **Peninsula Consulting Engineers**

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ABN 60 493 390 399

Ref # 21-0519- L001-REVA

14 July 2021

Mr Greg Mailman 212/20 Dale St, BROOKVALE NSW 2100

Flood Risk Assessment Report. Proposed Subdivision at <u>6 Brighton St Freshwater</u>

EXECUTIVE SUMMARY

Peninsula Consulting Engineers have been commissioned to undertake the required Flood Risk Assessment Report. This Report has relied upon the following documentation:

- Survey of proposed Subdivision prepared by Stutchbury Jaques Pty Ltd, dated 23rd November 2020, Ref 11008 and a Survey of Site by Stutchbury Jaques Pty Ltd, Job No.11008 of 15/02/2020. [refer Appendix A.]
- Survey of surrounding area prepared by Stutchbury Jaques Pty Ltd, dated 30th June 2021, Ref 11008/20 [refer Appendix A.]
- Flood Information provided by Northern Beaches Council available on Council website "Dee Why & Curl Cool Lagoons Floodplain Risk Management Plan" by Lyall & Associates, November 2005.
- The property & its immediate surrounds do not appear on Council's Flood Hazard Maps.

The premises have been assessed in accordance with the requirements of Warringah Councils LEP 2011 and Section E11 of the Warringah Development Control Plan (DCP), Council's Flood Advice information, and the NSW Government Floodplain Development Manual (April 2005.)

AIM OF REPORT

This report aims at investigating the water surface profile and flooding extent for the 100-year ARI (Average Recurrence Interval) Storm and outlines the methods used. The general information regarding pipe locations & sizes was obtained from Northern Beaches Council's Stormwater Map. This was overlaid in an Autocad drawing and scaled up, to show sub catchments. A *DRAINS* model was then used to determine corresponding catchment runoff and flows through the subject site. The HEC-RAS computer program was used to determine the 100-year ARI water surface profile, for the area around the subject site, for the existing situation and to determine the impact that the proposed development will have on the predicted



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overland flow path at the subject property and determine the most suitable method of alleviating the flooding issues on the site.

FLOOD INFORMATION OBTAINED FROM COUNCIL

Details of existing stormwater infrastructure and a contributing catchment area was assessed on site and obtained from Council webpage information and This information was utilised to determine catchment flows. The results were incorporated in the *DRAINS* model, with 100% pipe blockage used, to be conservative, for pipes in the catchment and through the subject site.

All calculations in this report have been prepared in accordance with Australian Rainfall and Runoff and Council's Drainage Manual.

DETAILS OF CATCHMENT AREA

The area is located between Brighton and Corella Street, to the west and Seaview Avenue to the east, and upstream from Brighton Street, to a summit in Corella Street, through Cooksey Avenue and up to Seaview Avenue, Freshwater. The Subdivision is of Lot A, DP359775.

EXISTING COUNCIL DRAINAGE INFRASTRUCTURE

The Pits upstream of the site were approximately 3 in number, and an 100% blocking factor was applied to the *DRAINS* calculations, on the pits upstream of the site. At the east side of the subject subdivision a 450mm, diameter Council Pipe conveys stormwater to the north, and then down Brighton Street to the west. The catchment area under assessment includes a catchment of 4.2 Hectares as shown on Appendix B. The inflow adjacent to the site was calculated to be 1.7m³/sec. this was assessed as overland flow in the subject area, and no allowance was made for pipe capacity. The numbering system of nodes corresponds with the *DRAINS* model on Appendix C.

Council did not have Flood Data for this catchment. The stormwater conveyance through the site was assessed from the survey mentioned above.

DESCRIPTION OF PROPOSED DEVELOPMENT & IMPACT ON LOCAL FLOOD BEHAVIOR

The proposed development is a subdivision with demolition of existing residence, and constuction of two residences, of mirror image, each on an individual block of land, with the new boundary being central to existing, and running north south. Refer Appendix A for plan of footprint of new residences on subdivision plan.

It was suggested to Council's Development Engineer by phone, on 7/07/21, that a solution to the flooding of the subject property would be to erect a boundary fence upon the eastern boundary, of sufficient height and bulk, to re-direct the flood



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waters into the Council Reserve adjacent to the site. This proposal was accepted verbally by Council, and consquently the proposed development will include this flood proof fence.

COUNCIL CRITERIA TO BE ADDRESSED

Council requires a minimum freeboard of 500mm above the predicted 1 in 100 year ARI water surface level for the floor of habitable rooms.

Council also require that no further impedance of flows are to be caused by the development, and flood storage volume is to be maintained within the site.

None of this was necessary as upstream flooding was eliminated on the site by introduction of the flood proof boundary fence.

HEC RAS RESULTS

The HEC RAS model was set up, with 9 cross sections to model flows coming into the site, and a 100 Year ARI inflow of 1.7m³/sec. Extent of 100Yr ARI flooding, both pre & post development are shown at Appendix D, together with numerical HEC RAS results. It was found that the boundary fence proposal would solve the flooding issues, without any restrictions on the floor elevations of the new development.

CONCLUSIONS

This residential development is satisfactory from a flood risk point of view. Key points are as follows.

- The provision of a waterproof reinforced masonry fence along the eastern boundary of the subject property, connecting to the front boundary fence of No. 22 Corella Street, and being at least 300mm in height above the 1% AEP top water level. Refer attached wall elevation.
- 2. This is to be constructed on the private property side of the boundary, at the Developer's cost. The structural Engineer is to design a suitable flood compatible wall off a suitable footing to resist the proposed flood loads.
- 3. This will ensure that flood flows from Corella Street and other public areas uphill from the subject property, will not affect the subject property in any storm up to & beyond the 1% AEP Storm.
- 4. No further requirements are necessary for the subject property, in respect of floor levels related to flooding.
- 5. The installation of a flood proof fence along the eastern boundary will improve the flooding outcomes for existing properties to the west of the subject site.



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REPORT'S AUTHOR & REVIEWER

This Report's Author and Reviewer have had a combined 70 years experience, in the engineering profession, including periods of employment in the Consulting, Government Instrumentalities, and Construction areas. The author's last full time employment was as Principal Technical Specialist [Stormwater] with Kellogg, Brown & Root, International Consulting Engineers. He carried out recent investigations for:

- Sydney Desalination Pipeline
- Victorian Highways
- Pilbara Railway
- NSW Railways

Peninsula Consulting Engineers has carried out Flood Risk Assessment Reports on a residential scale for Councils, Including Warringah, Pittwater, Willoughby, Wollongong, & Manly. Bruce Lewis, our Principal is - B.E. (Civil), CP Eng, NPER.

For any questions or queries on this report, please contact Peninsula Consulting Engineers.

Bruce Lewis Principal BE(Civil) Cpeng NPER Peninsula Consulting Engineers

Bruce Lewis (Reviewer) M: 0424 253 818 or E: Bruce@peninsulaconsutling.com.au.

Appendices

Appendix A – Proposed Subdivision & Survey Appendix B – Plan of Catchment Appendix C- *DRAINS* Model Appendix D – HEC RAS Results Appendix E – Proposed Fence Longitudinal Section.



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Appendix B Plan of Catchment





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Appendix C DRAINS Model





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Appendix D- HEC RAS Results Predevelopment



- Pre Development



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Hec Ras Cross Sections







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Ch 64.4- Flows around Existing Residence



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River Sta	Q Total	Min Ch El	W.S. Elev	Depth	Crit W.S.	E.G. Elev	Vel Chnl	Flow Area	Top Width	Froude # Chl
	(m ³ /s)	(m)	(m)	(m)	(m)	(m)	(m/s)	(m ²)	(m)	
182	0.69	36.51	36.64	0.13	36.71	36.96	2.49	0.28	5.68	3.6
120.6	1.14	29.8	29.96	0.16	30.13	31.26	5.04	0.23	2.89	5.6
87.6	1.14	25.97	26.49	0.52	26.68	27.11	3.49	0.33	1.35	2.27
64.4	1.14	21.28	21.42	0.14	21.55	22.38	0.25	0.26	3.89	0.3
36.4	1.74	19.12	19.27	0.15	19.32	19.45	3.06	1.26	22.53	3.58
20	1.74	17.85	18.05	0.2	18.16	18.6	3.3	0.55	8.69	4.03
0	1.74	16.92	17.14	0.22	17.25	17.6	3.01	0.59	6.86	3.15

HEC RAS Numerical Results Pre Development



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Appendix D - HEC RAS Results Post Development



Location of Cross Sections & Area of Inundation <u>– Post Development</u>



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Hec Ras Cross Sections







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River Sta	Q Total	Min Ch El	W.S. Elev	Depth	Crit W.S.	E.G. Elev	Vel Chnl	Flow Area	Top Width	Froude # Chl
	(m ³ /s)	(m)	(m)	(m)	(m)	(m)	(m/s)	(m ²)	(m)	
182	0.69	36.51	36.64	0.13	36.71	36.96	2.49	0.28	5.68	3.6
120.6	1.14	29.8	29.96	0.16	30.13	31.26	5.04	0.23	2.89	5.6
87.6	1.14	25.97	26.49	0.52	26.68	27.11	3.49	0.33	1.35	2.27
85.4	1.14	25.7	26.46	0.76	26.59	26.86	2.8	0.41	1.06	1.45
62.7	1.14	21.5	22.15	0.65	22.38	23.57	5.28	0.22	1.36	4.24
39.2	1.74	18.75	18.99	0.24	19.18	20.45	5.46	0.37	3.78	4.95
20	1.74	17.85	18.05	0.2	18.16	18.65	3.45	0.53	8.46	4.24
0	1.74	16.92	17.14	0.22	17.25	17.59	2.99	0.6	6.89	3.12

HEC RAS Numerical Results Post Development



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Appendix E – Proposed Fence – Longitudinal Section



CONCEPT PROPOSED WALL LONGITUDINAL SECTION

SCALE = NTS

